


BUILDING SCIENCE LIVE  
OCTOBER 27, 2021

## Balconies and Guards: "Safe-Guard" Your Railing Design

David C. Young | PE



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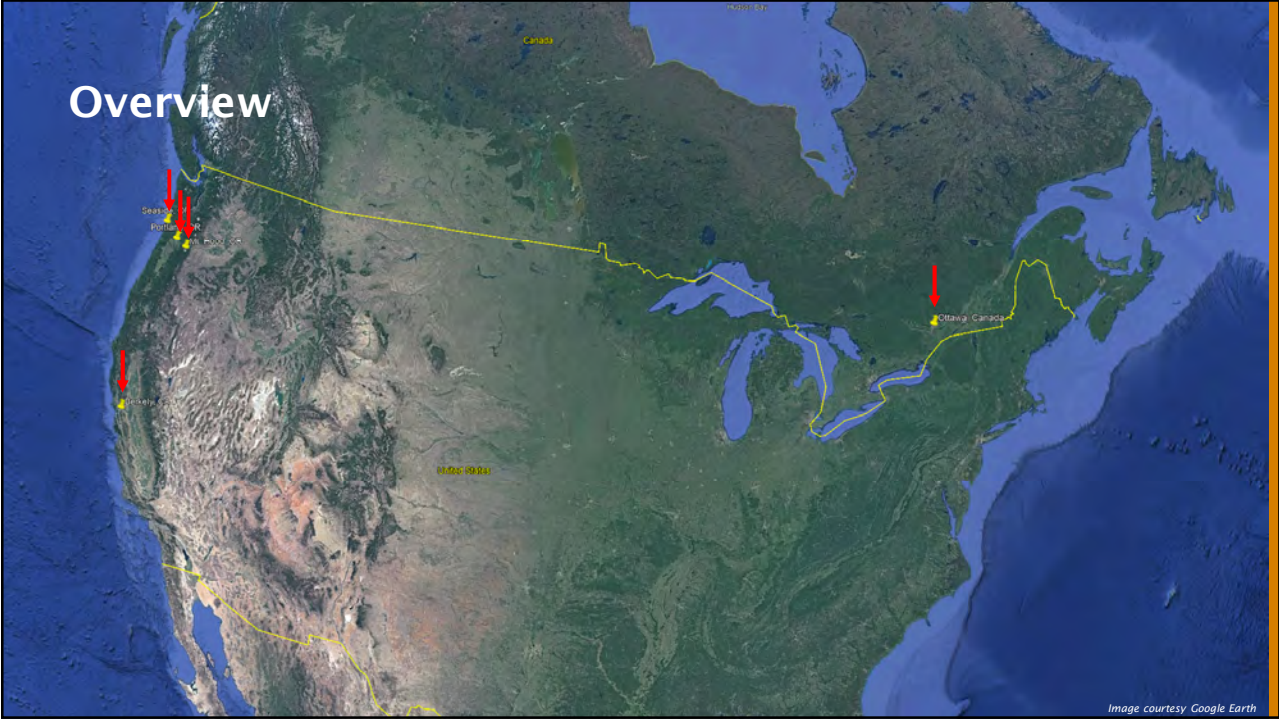
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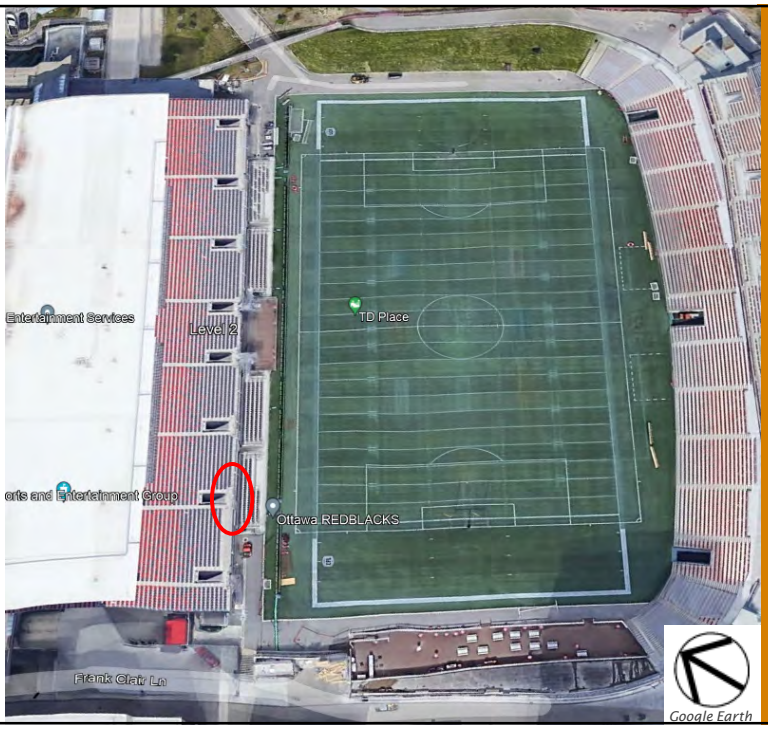
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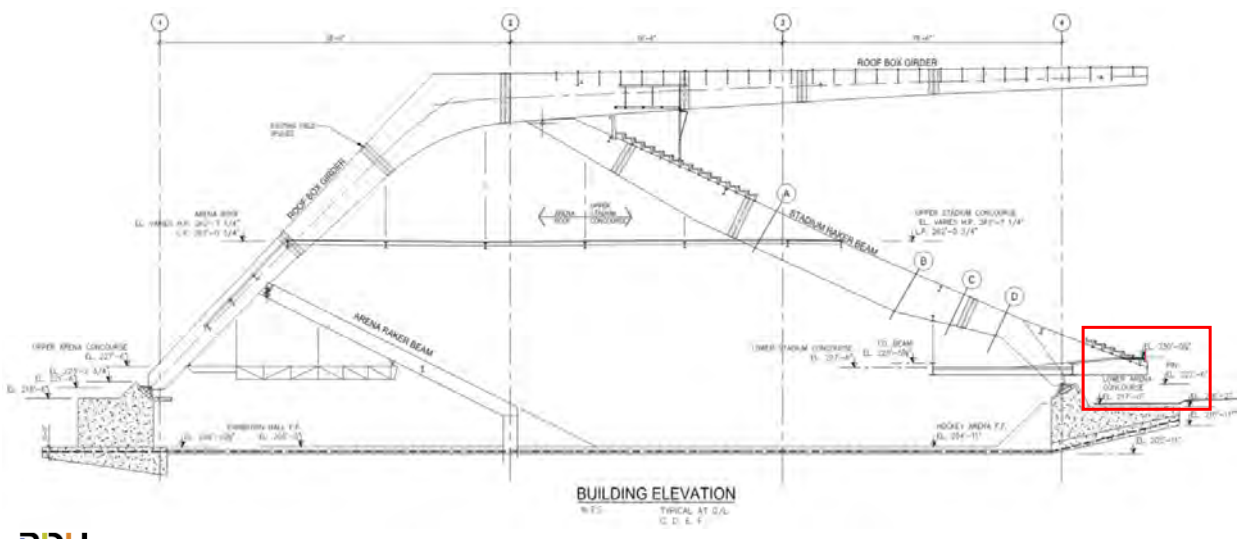
### Lansdowne Park Guardrail Failure

- North Stands completed in 1967 (TD Place)
- "Panda" Football Game Oct. 17, 1987 Carleton University (Ravens) vs Ottawa University (Braves)
- Half-time - large number of people crowding railing with notions of climbing and jumping onto the field
- 26ft of guardrail gave way around 2pm and 14 people fell over, 36 taken to hospital in total
- No fatalities



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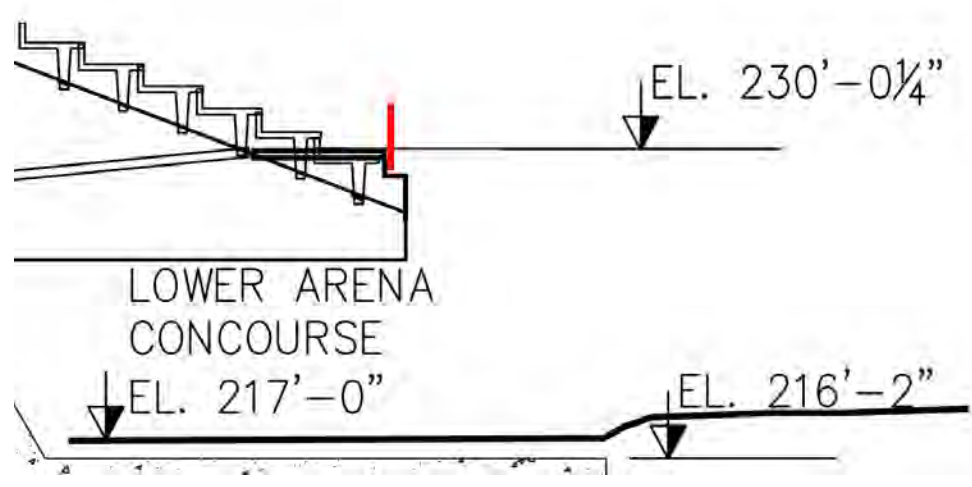
### TD Place – North Stadium



Graphic courtesy Adjeleian Allen Rubeli, Ltd.

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### TD Place - North Stadium

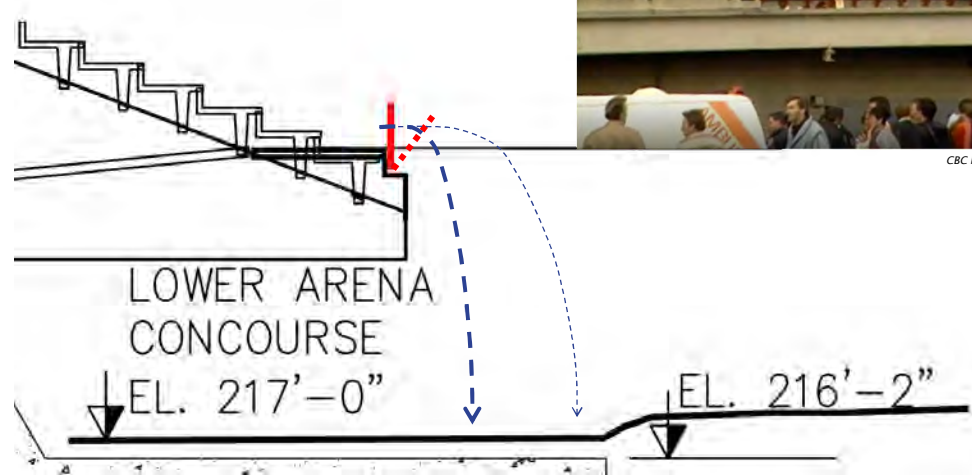


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*Graphic courtesy Adjeleian Allen Rubeli, Ltd.*

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### TD Place - North Stadium



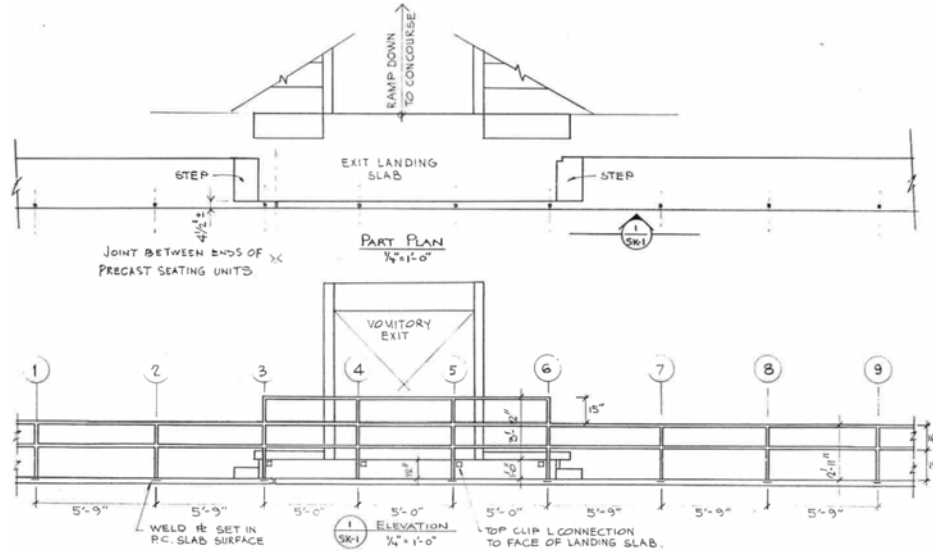
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*CBC News Archives*

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## TD Place – North Stadium



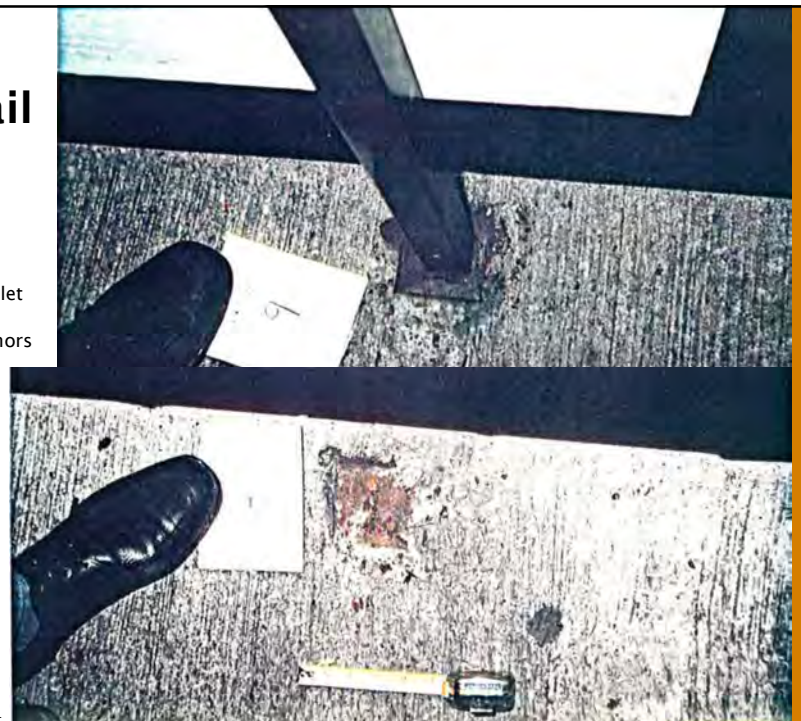
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## TD Place – Guardrail Failure

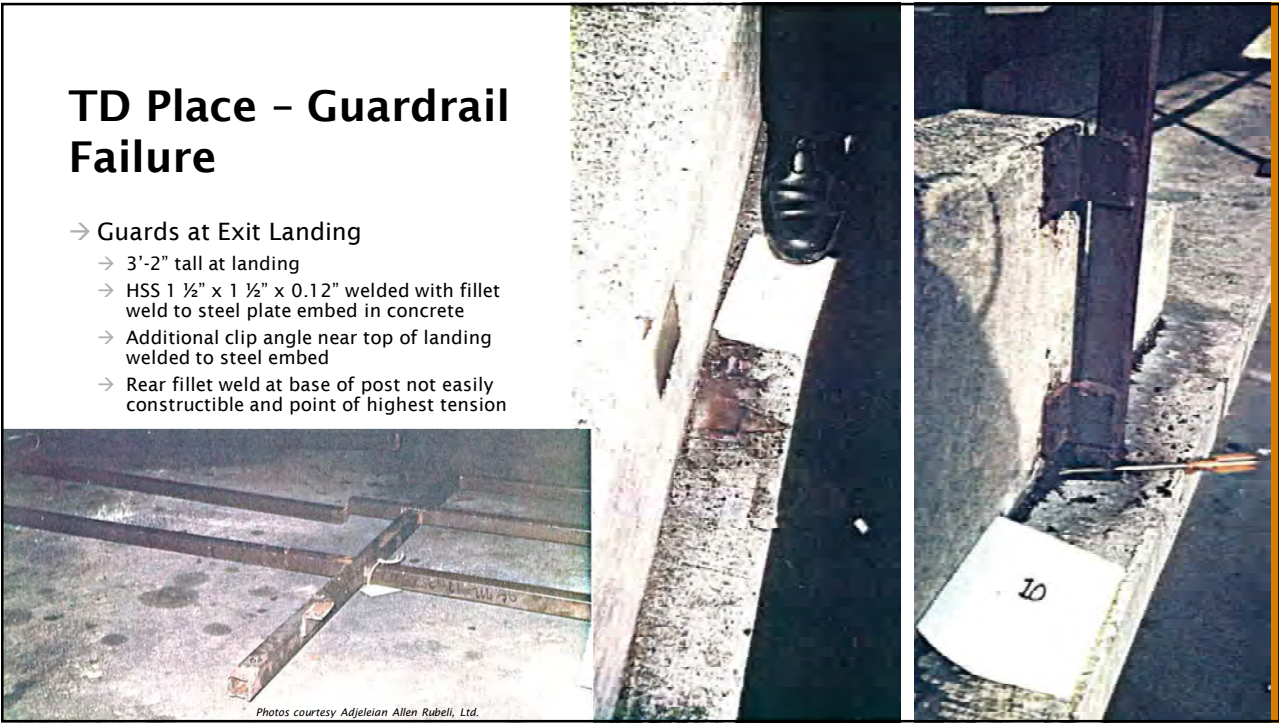
- Typical Railings at Seating
  - 2'-11" tall
  - HSS 1 1/2" x 1 1/2" x 0.12" welded with fillet weld to steel plate embed in concrete
  - Weld plate set with 3 headed stud anchors
  - Welds were incomplete seal welds which allowed water/condensation inside of post led to corrosion of the steel at the base of the post
  - Water was also able to penetrate under the weld plate leading to corrosion of the headed stud anchors
  - Not easily seen by visual inspection



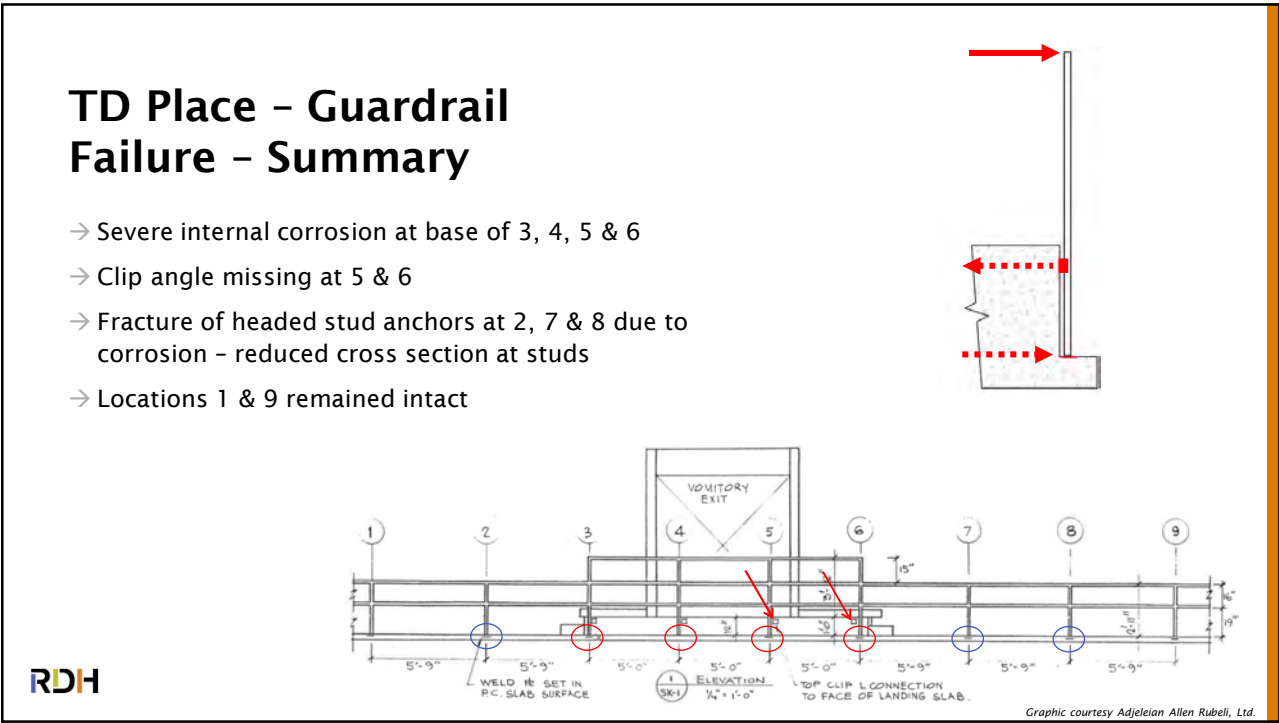
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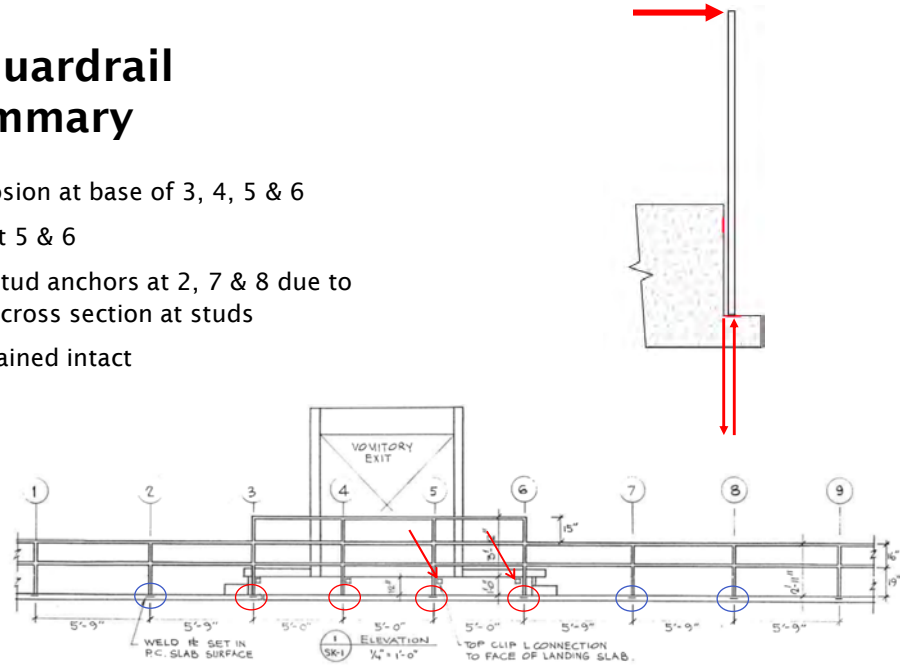
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## TD Place – Guardrail Failure – Summary

- Severe internal corrosion at base of 3, 4, 5 & 6
- Clip angle missing at 5 & 6
- Fracture of headed stud anchors at 2, 7 & 8 due to corrosion – reduced cross section at studs
- Locations 1 & 9 remained intact



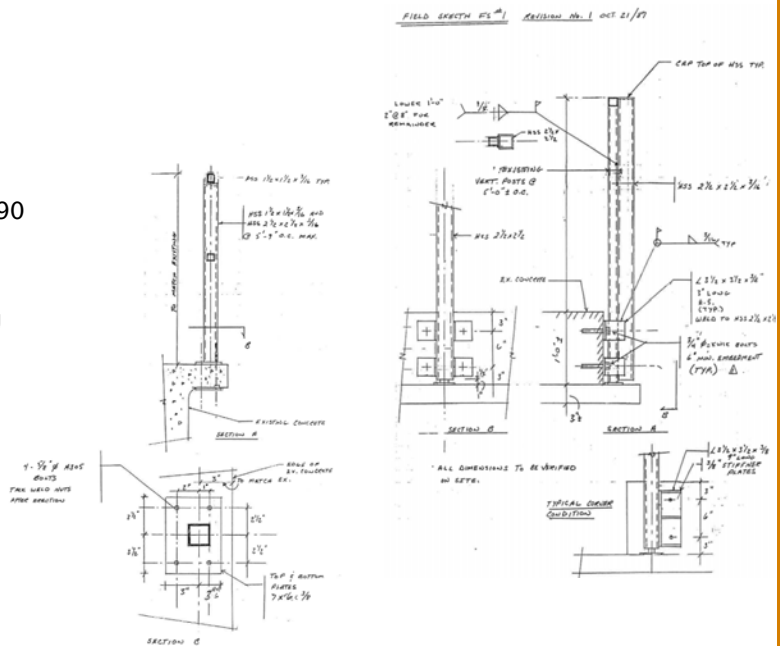
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Graphic courtesy Adjeleian Allen Rubeli, Ltd.

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## TD Place Guardrail Replacement

- Railing replacement started in 1990 after complete redesign
- All railings removed and replaced with heavier steel guards meeting Code requirements at that time
- Base of posts welded to steel base plates and through bolted to precast concrete or side bolted into thicker CIP.
- Many areas required concrete repairs



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Graphic courtesy Adjeleian Allen Rubeli, Ltd.

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## Library Gardens – Berkley, CA

- 5-story wood-framed apartment building, completed in 2006
- June 16, 2015, at 12:40am – balcony gave way
- 6 fatalities, 7 students suffered serious injuries
- Cantilevered balcony framing (LVL)
- OSB sheathing with self-adhered membrane waterproofing and concrete topping (alum. balcony-form edge)



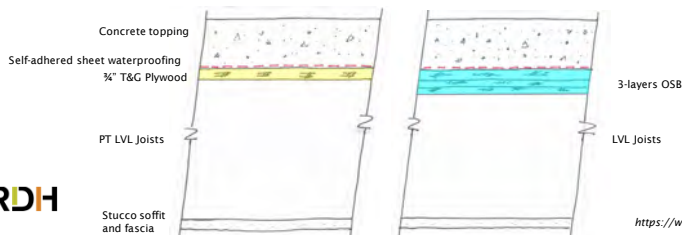
[https://www.cslb.ca.gov/Resources/Reports/Investigative/cslb\\_berkeley\\_balcony\\_materials\\_packet.pdf](https://www.cslb.ca.gov/Resources/Reports/Investigative/cslb_berkeley_balcony_materials_packet.pdf)



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## Library Gardens – Berkley, CA

- Some key deviations and field conditions
    - Original details called for 3/4" T&G plywood
    - As-built 3 layers of OSB sheathing (1 3/4" layer and 2 5/8" layers)
    - No membrane pre-stripping under edge extrusion, back lapped deck to wall membrane
    - Balcony framing completed mid-Oct. 2005
    - Waterproofing completed by Aug. 2, 2006
    - 38.78" of rain recorded over that period
- > 10-month exposure



**Trapped construction moisture with no ability to dry exacerbated by other poor perimeter detailing**

[https://www.cslb.ca.gov/Resources/Reports/Investigative/cslb\\_berkeley\\_balcony\\_materials\\_packet.pdf](https://www.cslb.ca.gov/Resources/Reports/Investigative/cslb_berkeley_balcony_materials_packet.pdf)



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## Library Gardens – Berkeley, CA – Outcomes

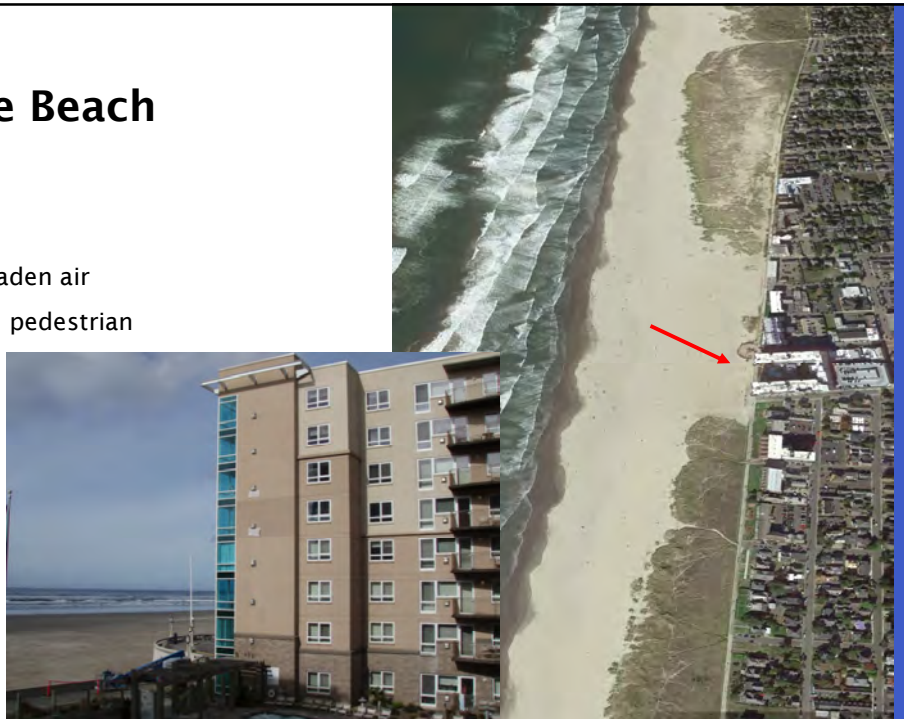
- California Building Code and Berkeley Municipal Code updated to include:
  - Section 1202.7 – Added vented space under balconies, decks, etc. (1/150<sup>th</sup> of area)
  - Sections 1404.13, 2304.11.1, and 2304.11.4.2 - Materials to be naturally durable or PT wood and corrosion resistant metal components
  - 601.4 Structural Maintenance
    - Essentially all elevated balconies, decks, stairs, landings, handrails and all parts of weather-exposed areas Group R-1 or R-2 to be inspected on 5-year interval.
- California Senate Bill 721 (2018)
  - **Apartments** (multi-family residential with 3 or more dwelling units)
  - Requires inspection of 15% of *Exterior Elevated Elements*
  - Review on 6-year intervals
- California Senate Bill 326 (2019)
  - **Condos**
  - Requires inspection of a “statistically significant sample” of *Exterior Elevated Elements*, for 95% confidence level,  $\pm 5\%$  margin of error
  - Review on 9-year intervals



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## Building at the Beach Seaside, OR

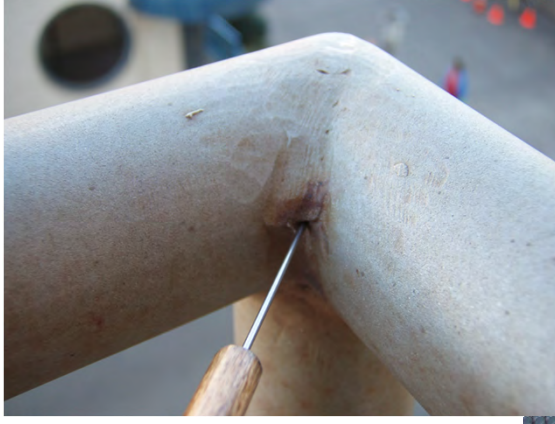
- 8-story resort hotel
- Full exposure and salt-laden air
- Concrete balconies with pedestrian coating and SS guards
  - 304 stainless
  - Set in grout pockets
- Corrosion on guards
- Spalling concrete at post locations



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# Building at the Beach Seaside, OR

→ Pinholes in welds allowed water into posts



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# Building at the Beach Seaside, OR

→ Coating had insufficient upturn/adhesion at posts



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### Building at the Beach Seaside, OR

- Grout pocket sleeves created additional pathway for water
- Water entering perimeter of post migrated to rebar and initiated corrosion



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### Building at the Beach Seaside, OR



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## Building at the Beach Seaside, OR

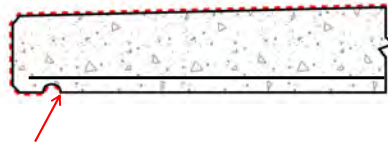
- In marine exposure, choose high corrosion resistant materials
  - 316 stainless or greater
  - Consider Duplex 2205 series stainless
- Avoid grout pockets
- Maintain adequate cover to reinforcing
- Detail coatings carefully and wrap to back of drip reglet, due to reduced rebar cover

### Key Components of Stainless Steels

|            | 304 | 316 | 2205  |
|------------|-----|-----|-------|
| Chromium   | 18% | 16% | 21%   |
| Nickel     | 8%  | 10% | 4.50% |
| Molybdenum |     | 2%  | 2.50% |

### Mechanical Properties

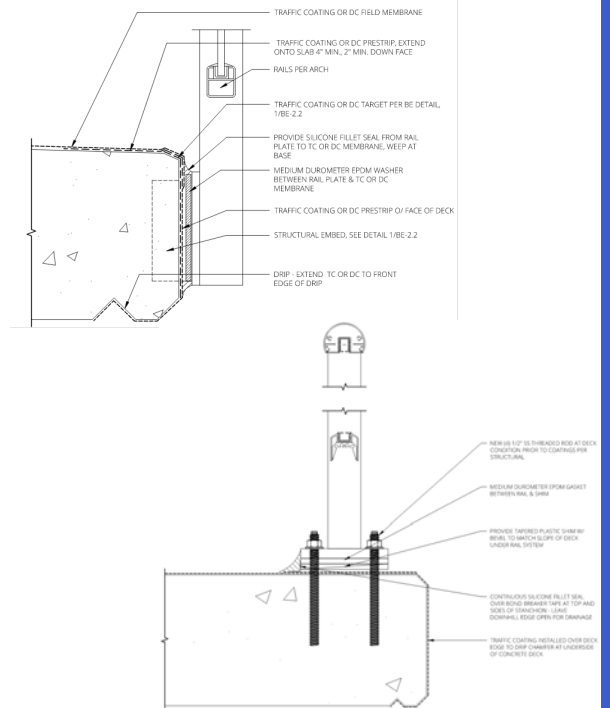
|                    | 304 | 316 | 2205 |
|--------------------|-----|-----|------|
| Ult. Tensile (Mpa) | 515 | 515 | 700  |



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## Some Design Approaches

- Coatings on concrete
  - Polyurethane membranes are very common - Typically require recoating every 5-8 years
  - Consider PMMA or PUMA - require recoating in the range of 10-15 years.
    - Higher install cost, lower life cycle cost
- Top or side mounted guards for concrete decks



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## Some Design Approaches

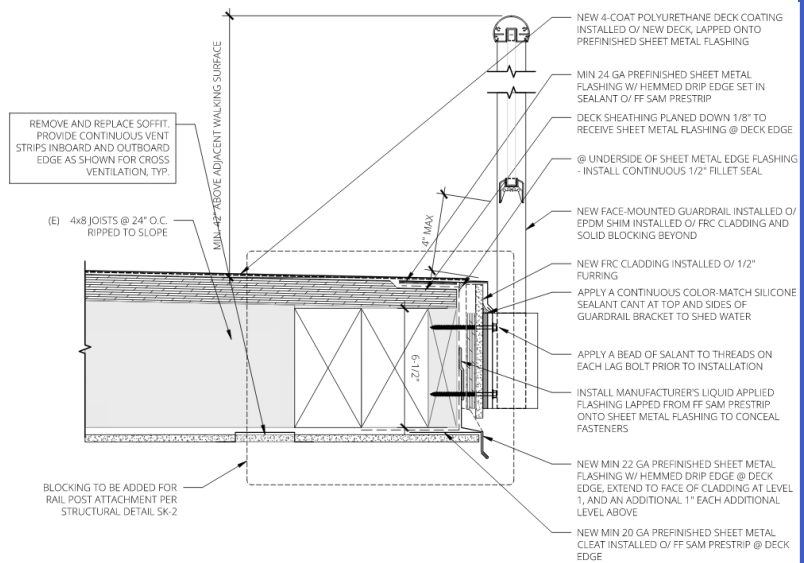
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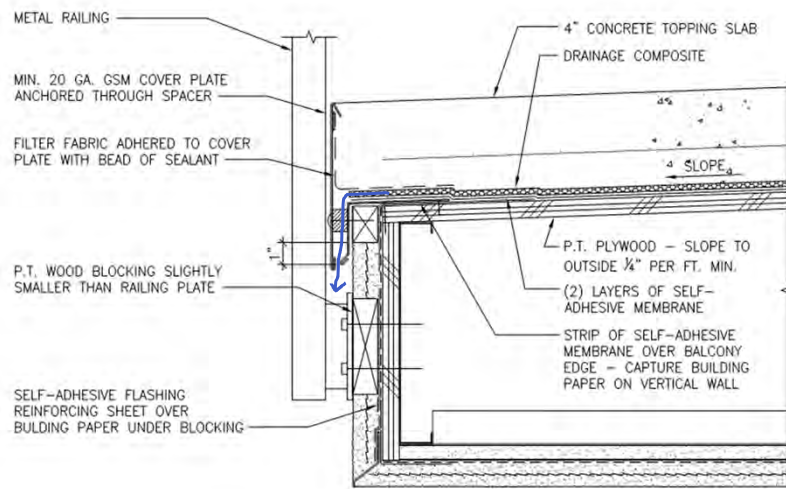
## Some Design Approaches

- Side mounted guards in wood framed const. Avoid top mounted guards wherever possible



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## Some Design Approaches



- Provide a clear means of drainage in concrete-topped wood-framed balconies (and rainscreen fascia)



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## Freeze-Thaw Damage

- What is Freeze-thaw?
  - A porous material freezes then thaws out
  - Water within pores freezes and expands, leading to deterioration
  - Common in brick, stone, concrete, manufactured products (hardi cement board, cultured stone etc.)
- Thresholds
  - Area of material must be near saturation moisture level for freezing to cause damage (75% to 95% of free water saturation)
  - Pore size/air voids matter (ie air-entrained concrete)
  - Temperature for freezing of water is 0°C, but F/T damage typically only occurs when freezing down below -5°C
- Number of F/T cycles until damage & resistance varies



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## Portland Condo – Balcony Guards

- Freeze/Thaw can also manifest in other ways
- Example of split railing – Photo taken in January 2009



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## Portland Condo – Balcony Guards

- Joints in railing caps and post connections allowed water to enter aluminum railing system



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## Portland Condo – Balcony Guards

- Example of bowed railing post – deformation as a result of frozen water and lack of drainage (photo taken August 2011).
- Condition observed at several balconies



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## Portland Condo – Balcony Guards

- Damaged railing post cut and removed during January 2009 repair work
- Solid grout column at railing post location after removal during the repair work in 2009



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## Portland Condo – Balcony Guards

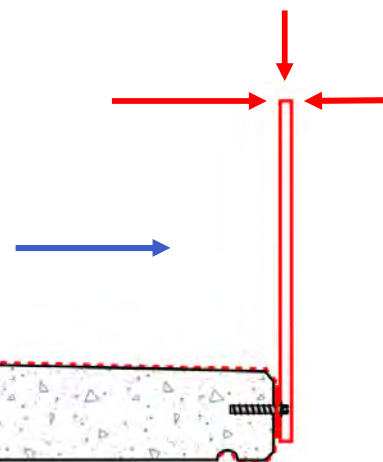
- Weep holes provided after 2009 event. Staining and residue from drainage at weep hole. Condition observed at most balconies and deck guards during August 2011
- Example of water released from weep hole after removal of a build-up of residue that blocked the weep hole. Observed at numerous locations.
- Solution – provide adequate means of drainage



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## General Design Loads for Guards

- **Canada**
  - Viewing Stands (Stadia) – 205plf (3kN/m) horiz. outward
  - Access ways to equipment platforms – 225lb (1kN) horiz. outward
  - All others – 50plf (0.75kN/m) horiz. Outward
  - Minimum inward – 50% of above loading
  - UDL – 100plf (1.5kN/m) applied vertically, or
  - Point Load – 1.0kN applied vertically
  - Components – 100lb (0.5kN) horiz. over 12" x12" areas
  - Consult CSA A500-16 – Building Guards when designing railings, particularly glass guards (Doesn't include Assembly occupancies)
- **US**
  - No special provision for Viewing Stands
  - All top rails – 50plf (0.73kN/m) – applied in any direction
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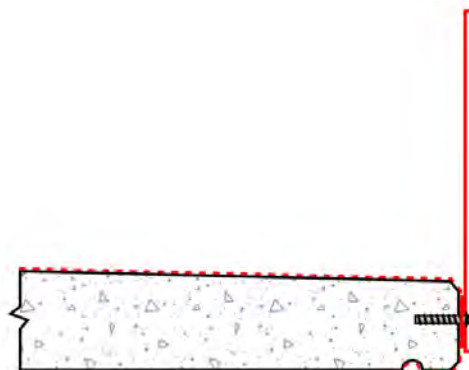


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## General Design Loads for Guards

→ Canada

→ US



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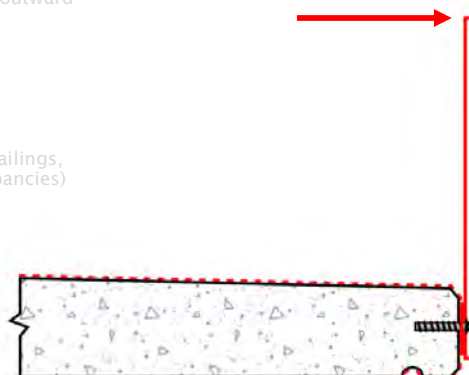
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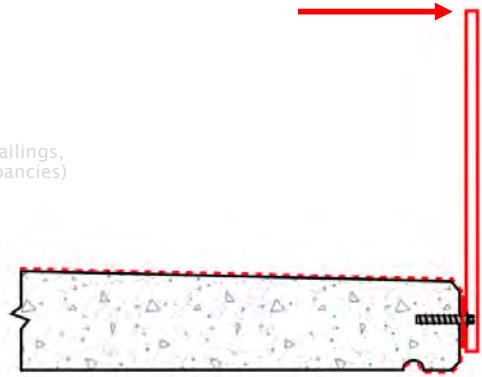
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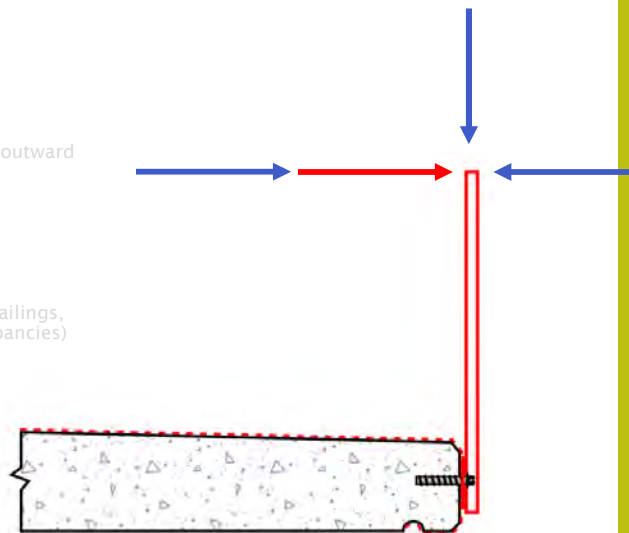
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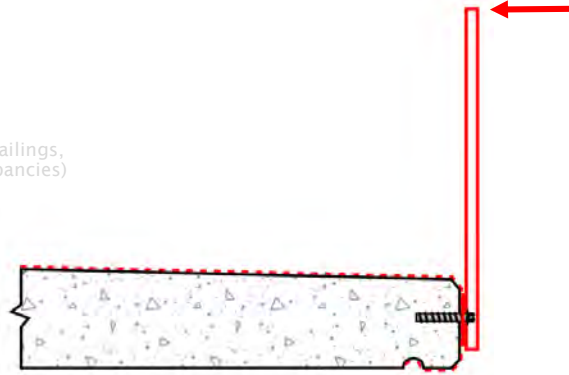
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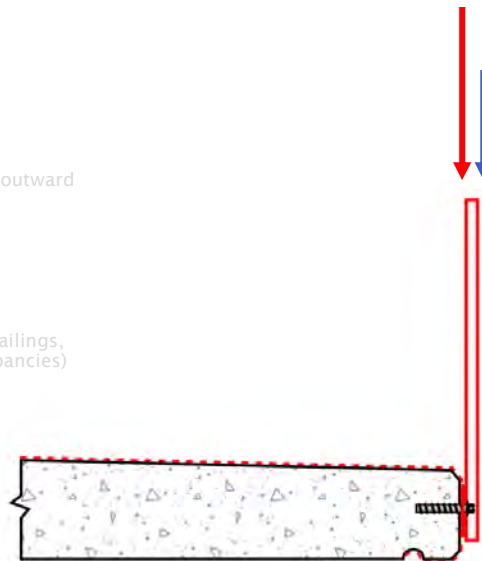
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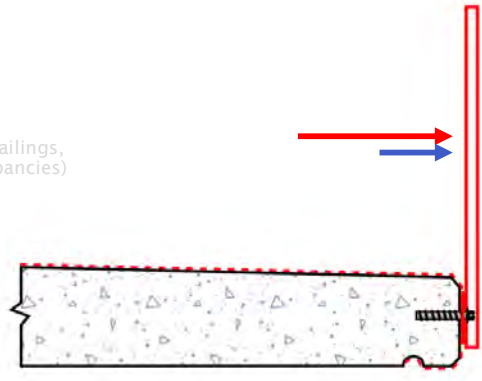
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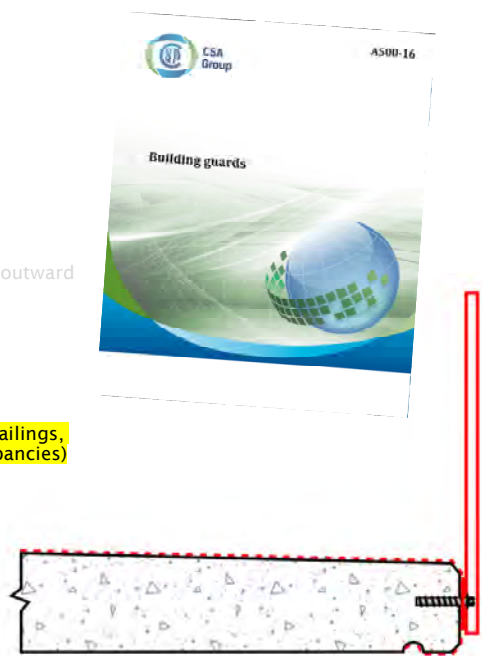
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## General Design Loads for Balcony Surface

→ **Canada**  
→ 100psf (4.8kPa)

→ **US**  
→ 1.5 x area served  $\leq$  100psf (4.8kPa)  
→ Residential balconies = 40psf x 1.5 = 60psf (2.9kPa)

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## Don't forget Snow

→ Sometimes snow controls!

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# Don't forget Snow

- 3-story townhome style condos on Mt. Hood
- Elev. 3,800ft
- High snow load area
- Top mounted guards to wood framed drip through balconies



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## Thinking Outside the Box

- Strengthen Guards and Balconies to handle sliding snow???
- Shed snow away from balconies
- New cantilevered roof
- New side-mounted guards with decorative fascia



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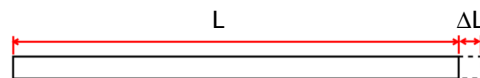
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## Thermal Expansion and Contraction of Materials

- Materials expand and contract with changes in temperature
- Important for the design of structural and enclosure components, as well as aluminum guards

$$\rightarrow \Delta L = \alpha \cdot L \cdot \Delta T$$

- Where  $\Delta L$  = material length change,  $L$  is initial length,  $\alpha$  is thermal expansion/contraction coefficient,  $\Delta T$  is change in temperature



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## Material Properties (SI)

- Steel,  $12 \times 10^{-6}$
- Concrete,  $12 \times 10^{-6}$
- Aluminum,  $24 \times 10^{-6}$
- Lead,  $30 \times 10^{-6}$

Table 13.1: Properties needed to assess change in dimension of materials

| Material*                               | Coefficient of Thermal Expansion and Contraction (α) / Ca <sup>10<sup>-6</sup></sup> | Coefficient of Expansion and Contraction Due to Moisture Change (α) % | Irreversible Moisture Movement (+) Expansion (-) Shrinkage %  | Modulus of Elasticity (E) , kN/mm <sup>2</sup> |
|---|--|---|---|--|
| <b>Natural Stone</b>                    |  |   |   |  |
| Granite                                 | 8-10   |   |   | 20-60  |
| Limestone                               | 3-4  | 0.01  |   | 10-60  |
| Marble                                  | 4-6  |   |   | 35   |
| Sandstone                               | 7-12   | 0.07  |   | 3-50   |
| Slate                                   | 9-11   |   |   | 10-35  |
| Stellar and Fine Concrete               | 10-13  | 0.02-0.08   | 0.04-0.10(-)  | 20-35  |
| Dense Aggregate Concrete                |  |   |   |  |
| Gravel Aggregate                        | 12-14  | 0.02-0.06   | 0.03-0.08(-)  | 16-36  |
| Crushed Rock (Except Limestone)         | 10-13  | 0.03-0.10   | 0.03-0.08(-)  | 16-36  |
| <b>Common/Ready-Mix Concrete</b>        |  |   |   |  |
| Limestone                               | 7-8  | 0.02-0.03   | 0.03-0.04(-)  | 20-35  |
| Steel-Fiber-Reinforced Concrete         | 5-14   | 0.02-0.06   | 0.03-0.06(-)  | 20-41  |
| Aerated Concrete                        | 8  | 0.02-0.03   | 0.07-0.09(-)  | 1.4-3.2  |
| Lightweight Aggregate Concrete          |  |   |   |  |
| Medium Lightweight                      | 8-12   | 0.03-0.06   | 0.03-0.09 (-)   | 8  |
| Ultra Lightweight (Perlite)             | 8-9  | 0.10-0.20   | 0.20-0.40(-)  | 8  |
| Asbestos-Cement                         | 8-12   | 0.10-0.25   | 0.08(-)   | 14-26  |
| Glass-Reinforced Cement                 | 7-12   | 0.15-0.25   | 0.07(-)   | 20-34  |
| Asbestos Wallboard                      | 6-12   | 0.14-0.27   |   | 8-10   |
| Asbestos Insulation Wallboard           | 2.5-7.2  | 0.16-0.25   |   | 2 kN/m <sup>2</sup>                            |
| <b>Gypsum Board Composite</b>           |  |   |   |  |
| Dense Plaster: Plasterboard             | 18-21  |   | (gypsum materials are highly susceptible to moisture damage. Moisture changes are not reversible)   | 16   |
| Sanded Plaster                          | 12-15  |   |   | 8.5-16   |
| Lightweight Plaster                     | 16-18  |   |   | 1.5-4  |
| Glass-Reinforced Gypsum                 | 17-20  |   |   | 16-20  |
| <b>Brickwork, Blockwork, and Tiling</b> |  |   |   |  |
| Concrete Brickwork and Blockwork        |  |   |   |  |
| Dense Aggregate                         | 8-12   | 0.02-0.04   | 0.02-0.04(-)  | 10-25  |
| Lightweight Aggregate (Autoclave)       | 8-12   | 0.03-0.06   | 0.02-0.04(-)  | 4-18   |
| Aerated (Autoclave)                     | 8  | 0.02-0.03   | 0.05-0.09(-)  | 5-8  |
| Calcium Silicate Brickwork              | 8-14   | 0.01-0.05   | 0.01-0.04(-)  | 14-18  |
| Clay or Shale Brickwork or Blockwork    | 5-8  | 0.02  | 0.02-0.07(-)  | 4-26   |
| Clay Tiling                             | 4-8  |   |   | 8  |
| Cast Iron                               | 10   |   |   | 80-120   |
| Plain Carbon Steel                      | 12   |   | (While not subject to expansion or contraction due to moisture changes, some metals can be adversely affected by changes in moisture resulting in corrosion. Most metals will exhibit some change in appearance upon exposure to moisture changes.) | 210  |
| Stainless Steel                         |  |   |   |  |
| Austenitic                              | 18   |   |   | 200  |
| Ferritic                                | 10   |   |   | 200  |
| Aluminum and Alloys                     | 24   |   |   | 70   |
| Copper                                  | 17   |   |   | 85-130   |
| Bronze                                  | 20   |   |   | 100  |
| Aluminum Bronze                         | 18   |   |   | 120  |
| Brass                                   | 21   |   |   | 100  |
| Zinc                                    |  |   |   |  |
| Parallel to Rolling                     | 33   |   |   | 140  |
| Perpendicular to Rolling                | 23   |   |   | 220  |
| Lead                                    | 30   |   |   | 14   |

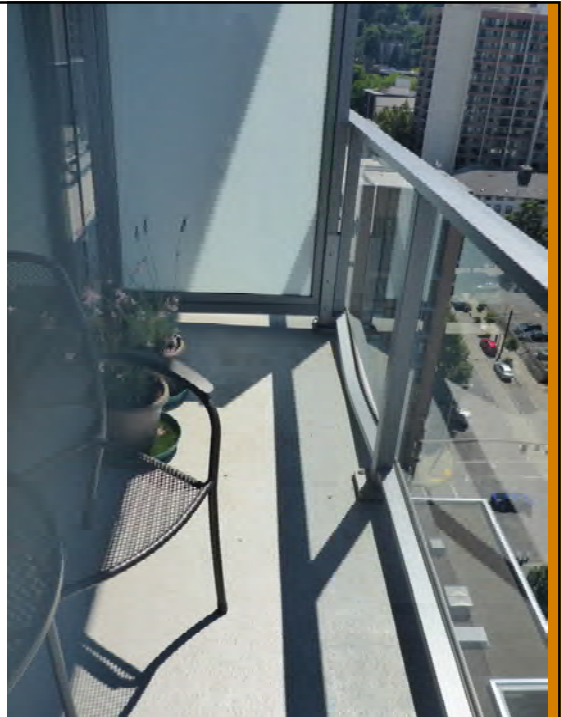
Straube & Burnett



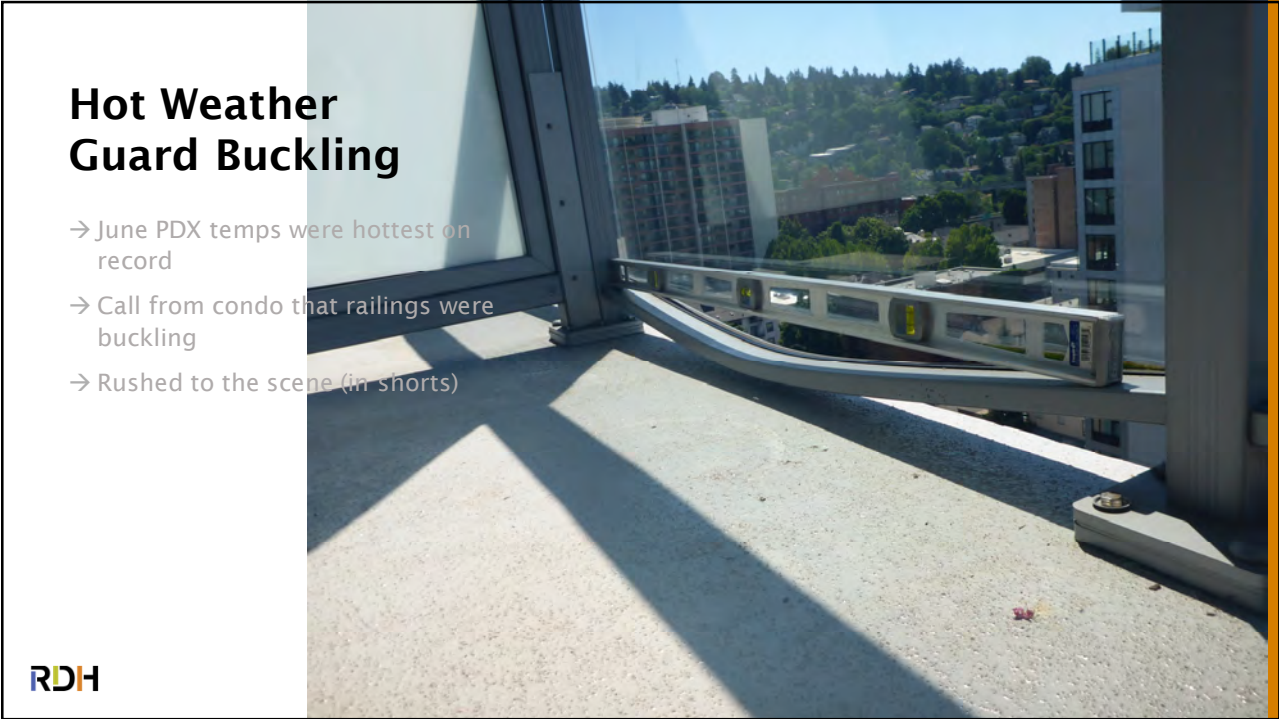
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## Hot Weather Guard Buckling

- Portland temperatures in June 2021 were hottest on record
- Received a call from condo board that railings were buckling
- Rushed to the scene (in shorts)



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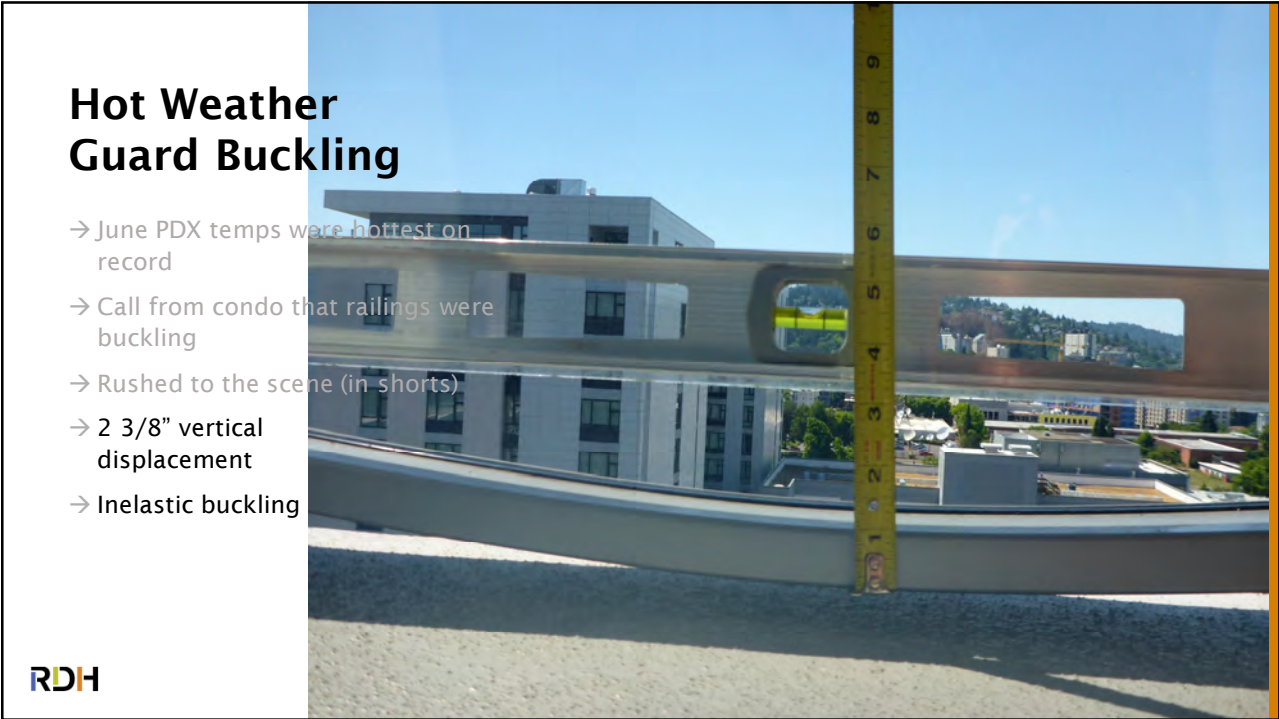


# Hot Weather Guard Buckling

- June PDX temps were hottest on record
- Call from condo that railings were buckling
- Rushed to the scene (in shorts)



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# Hot Weather Guard Buckling

- June PDX temps were hottest on record
- Call from condo that railings were buckling
- Rushed to the scene (in shorts)
- 2 3/8" vertical displacement
- Inelastic buckling



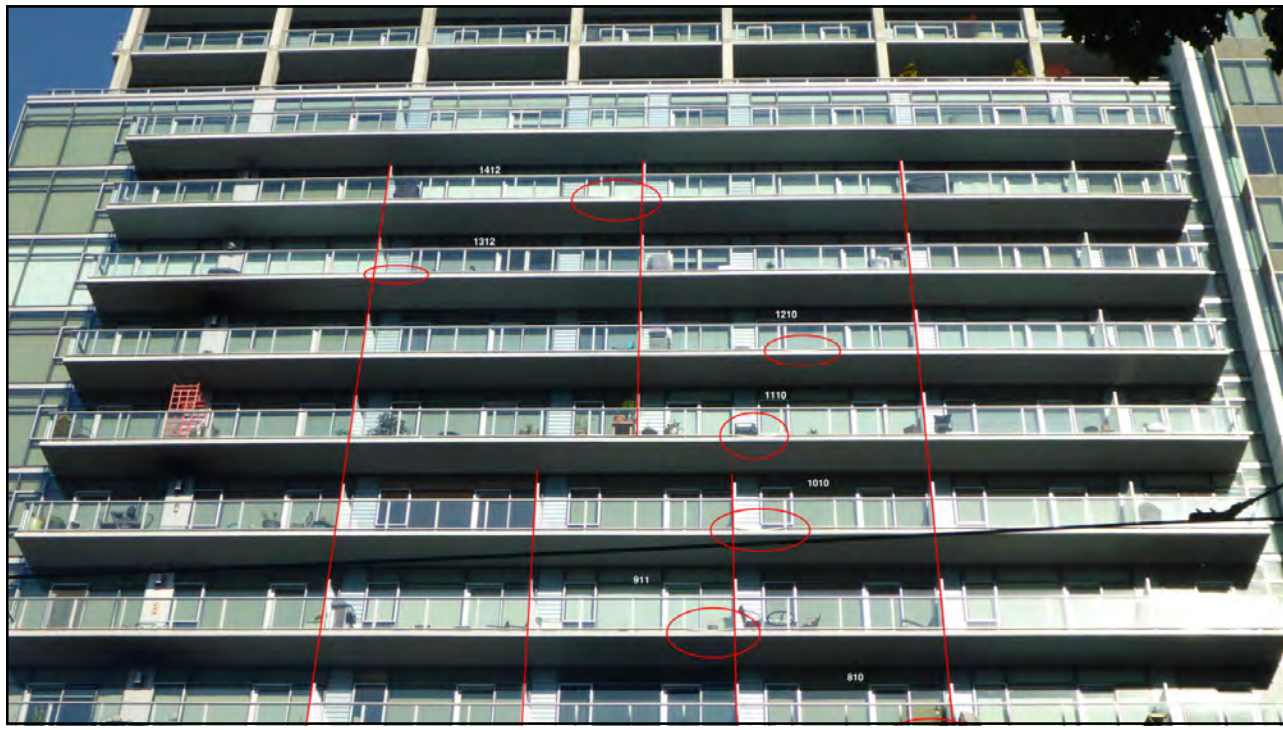
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# Hot Weather Guard Buckling

- Some bottom rails only experienced elastic deflection and did not buckle
- Other bottom rails were not affected



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## Hot Weather Guard Buckling

- Continuous Aluminum top rail on west elevation
- Top rails did not buckle
- Generally random locations of inelastic buckling of bottom rails, but mostly within center 1/3<sup>rd</sup> of elevation
- Typically, one inelastic buckling location per floor
- Several bottom rails deflected elastically (returned after cooling)



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## Portland Design Climate

- ASHRAE Fundamentals - 2017 (IP)
- PDX International Airport

Table 1.1-1 General Annual Climate Conditions

| Extreme Annual WS |      |      | Extreme Annual Temperature |      |                    |     | n-Year Return Period Values of Extreme Temperature |      |            |      |            |      |            |     |       |
|-------------------|------|------|----------------------------|------|--------------------|-----|--|------|------------|------|------------|------|------------|-----|-------|
|                   |      |      | Mean                       |      | Standard deviation |     | n=5 years  |      | n=10 years |      | n=20 years |      | n=50 years |     |       |
| 1%                | 2.5% | 5%   | Min                        | Max  | Min                | Max | Min  | Max  | Min        | Max  | Min        | Max  | Min        | Max |       |
| 23.7              | 19.7 | 17.4 | DB                         | 20.8 | 99.0               | 5.2 | 3.3  | 17.1 | 101.4      | 14.0 | 103.3      | 11.0 | 105.1      | 7.2 | 107.5 |
|                   |      |      | WB                         | 19.1 | 72.8               | 5.2 | 1.8  | 15.3 | 74.1       | 12.3 | 75.2       | 9.3  | 76.2       | 5.5 | 77.6  |

- High Temp on June 28, 2021 - 116 °F at 3:53pm
- 109 °F at time of review - 2:51pm
- High Temp > 8°F above 50-year return period extreme



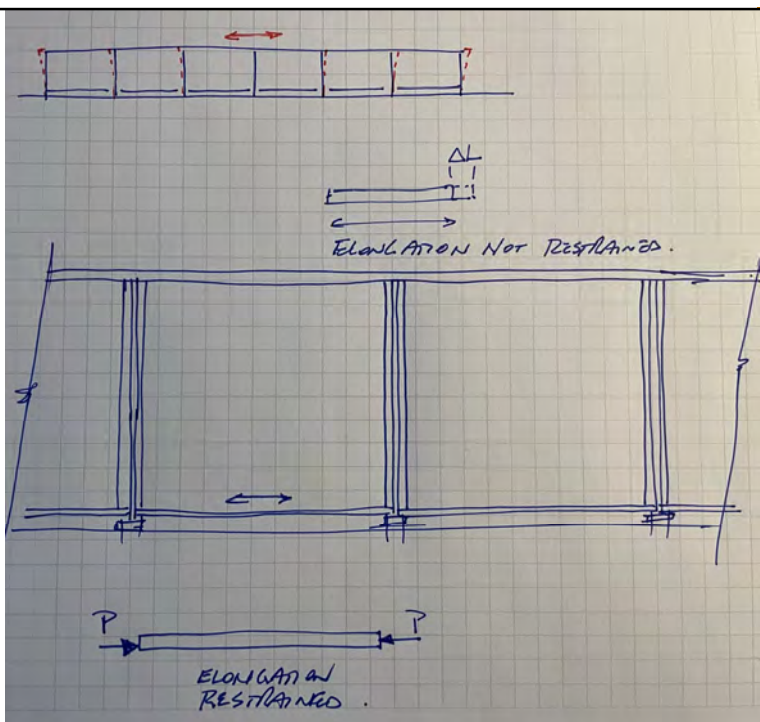
WMO# World Meteorological Organization number    Lat Latitude, °  
 Elev Elevation, m    SLP Standard pressure at station elevation, kPa    Long Longitude, °  
 DB Dry bulb temperature, °C    DP Dew point temperature, °C    WB Wet bulb temperature, °C  
 WS Wind speed, m/s    Enth Enthalpy, kJ/kg    HR Humidity ratio, grams of moisture per kilogram of dry air  
 MCB Mean coincident dry bulb temperature, °C    MCWB Mean coincident wet bulb temperature, °C    MCWS Mean coincident wind speed, m/s  
 PCWD Prevailing coincident wind direction, °, 0 = North, 90 = East

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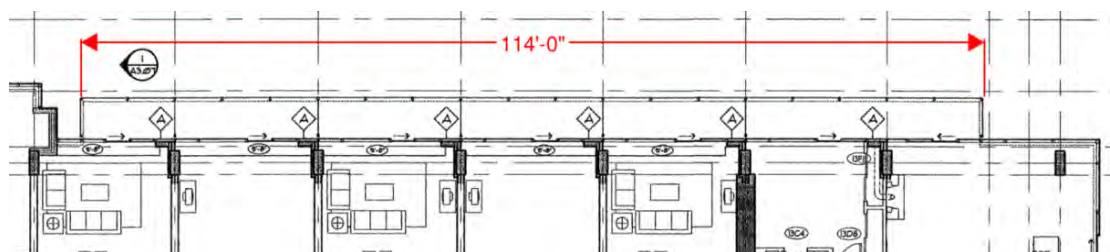
## Why Just Bottom?

- Top rail not restrained
- Bottom rail tightly restrained
- $\Delta L = \alpha \cdot L \cdot \Delta T$
- $\Delta L = \frac{(P \cdot L)}{(A \cdot E)}$ 
  - Where  $\Delta L$  = material length change, L is initial length, A is cross-sectional area, E is modulus of elasticity

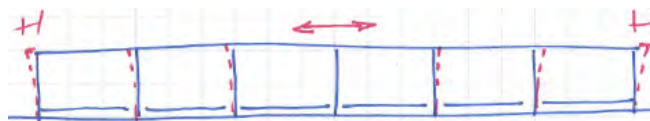


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## Top Rail Movement



- $\Delta L = \alpha \cdot L \cdot \Delta T \approx 1.8''$  total
  - L = 114ft = 1368in
  - $\alpha = 13 \times 10^{-6}$  in/in °F
  - Assumed install temp approx.  $\approx 65^\circ\text{F}$
  - Sol-Air Temp approx.  $\approx 50^\circ\text{F}$
  - $\Delta T = (116^\circ\text{F} - 65^\circ\text{F} + 50^\circ\text{F}) \approx 101^\circ\text{F}$

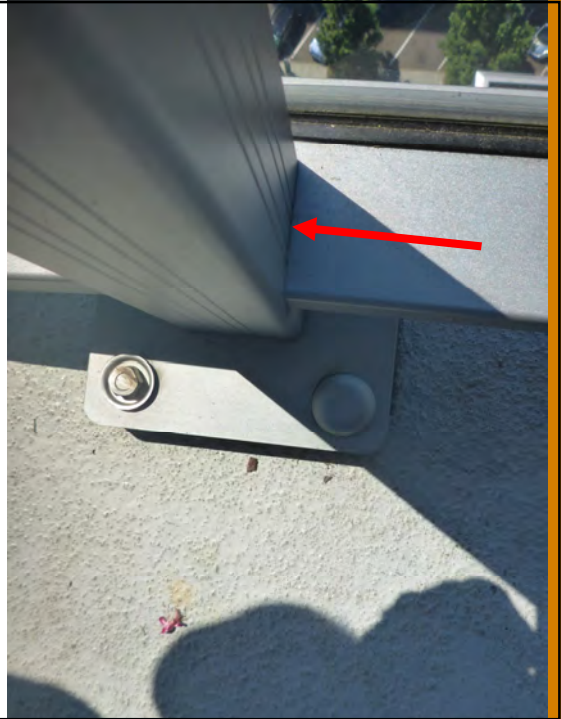


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## Bottom Rail

- Constrained by posts bolted directly to concrete
- Concrete  $\alpha = 7.8 \times 10^{-6}$  in/in $^{\circ}$ F
- Aluminum  $\alpha = 13 \times 10^{-6}$  in/in $^{\circ}$ F (almost double  $\alpha$  concrete)
- $\Delta L = \alpha \cdot L \cdot \Delta T \approx 1/16$  in total
  - $L \approx 52$ in
  - $\alpha = 13 \times 10^{-6}$  in/in $^{\circ}$ F
  - Assumed install temp approx.  $\approx 65$   $^{\circ}$ F
  - Sol-Air Temp approx.  $\approx 50$ F
  - $\Delta T = (116$   $^{\circ}$ F  $- 65$   $^{\circ}$ F  $+ 50$   $^{\circ}$ F)  $\approx 101$   $^{\circ}$ F
- Need to allow for movement!

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## Summary

- Design for long-term durability
- Consider constructability (avoid impossible welds, etc.)
- Protect weld plates, embeds, and materials susceptible to corrosion
- Protect moisture susceptible materials during construction
- Choose materials and assemblies appropriate for the climate, exposure, and application
- Detail to deflect, drain, and dry moisture away from guards, connections and adjacent surfaces
- Avoid grout pockets!
- Design in tolerance for the extremes
- Field review is always important

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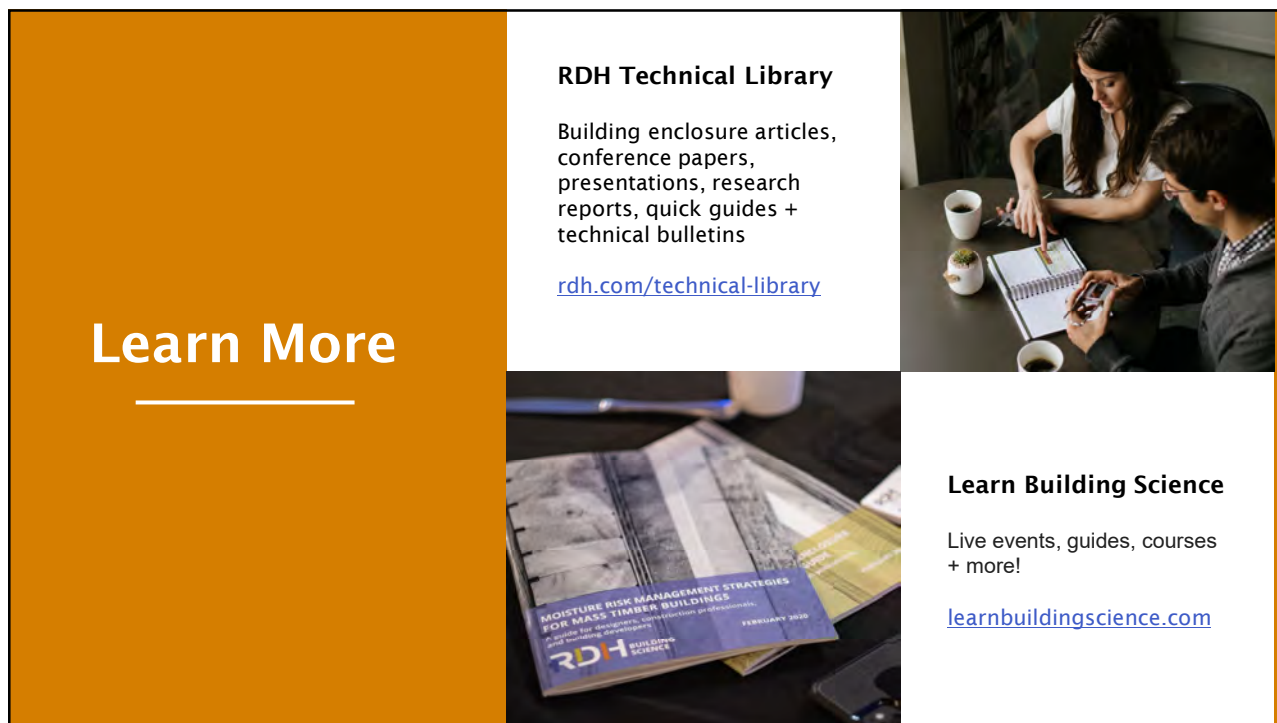
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 Time: Event opens at 8:00 am EDT and concludes at 5:30 pm EDT

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